

Calculation of parameters of hydrodynamically stable earth ducts

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Abstract: The article describes the calculation of the parameters of soil-channel channels with hydrodynamic stability. It provides formulas to calculate the width and

depth of water in large channels with hydrodynamically strong earth channels using existing studies.

Key words: stable channel, hydrodynamic stability, relative width, transport capacity, planned sustainability.

C – Shezy coefficient, assuming $L_m=8B$, B.C. Altunin [1] proposed a calculated dependence:

$$\frac{C}{\sqrt{g}} = \sqrt{0,2 \left(\frac{B}{h_{cp}} \right)^2 + 43}, \quad (4)$$

which for a given B / h_{av} allows to calculate C and, conversely, with a known C makes it possible to determine the relative width B / h_{av} .

In [5, 6], as a result of solving the problem of hydrodynamic stability of the channel, taking into account its transporting ability, an expression was obtained for the initial length of the meanders, which, assuming the conditions of the clarified flow, takes the form:

$$L_m = \frac{\pi C^2 h_{cp}}{g} \quad (5)$$

Accepting $L_m=30$ As a characteristic scale at which the condition of flow quasi-stability is maintained, we have obtained a dependence for calculating the relative width of stable earth channels in sandy unbound soils in the form:

$$\frac{B}{h_{cp}} = 0,1 \frac{C^2}{g} \quad (6)$$

Later, as a result of analyzing the existing regime dependences using the dimensional method, we obtained dependences for calculating the width and average depth of a dynamically stable channel, which are in fairly good agreement with empirical data (Figs. 1, 2):

$$B = 7,76 d_{cp} Q^{0,440} \quad npu \quad Q \geq 10^{11};$$

where M – Boussinesq parameter ($M = 22+24 M^{0,5/c}$);

K – parameter of the logarithmic velocity distribution function;

h_{cp} – average flow depth;

$$B=0,0023 d_{cp} Q^{-0,760} \text{ при } 10^{10} \leq Q < 10^{11}; \quad (7)$$

$$B=0,30 d_{cp} Q^{-0,550} \text{ при } 10^9 \leq Q < 10^{10};$$

$$B=88 d_{cp} Q^{-0,275} \text{ при } Q < 10^9;$$

$$h_{cp}=0,04 d_{cp} Q^{-0,503} \text{ при } Q > 10^{11};$$

$$h_{cp}=10,4 d_{cp} Q^{-0,281} \text{ при } 10^9 \leq Q \leq 10^{11}; \quad (8)$$

$$h_{cp}=0,19 d_{cp} Q^{-0,475} \text{ при } Q < 10^9;$$

где

$$Q = \frac{Q}{d_{cp}^2 \sqrt{gd_{cp}(S-1)}}$$

The choice of the calculation formula for the channel width is carried out on the basis of comparing the calculated, laboratory and field data. Dependences (1), (2), (4), (6) - (8) were tested as formulas. The results of comparison of actual and calculated data are presented in Table 1. The analysis shows that dependences (7) and (6) give the best agreement with the actual data, and (7) gives more reliable values. This made it possible to recommend it as a calculated dependence for determining the channel width over the free surface.

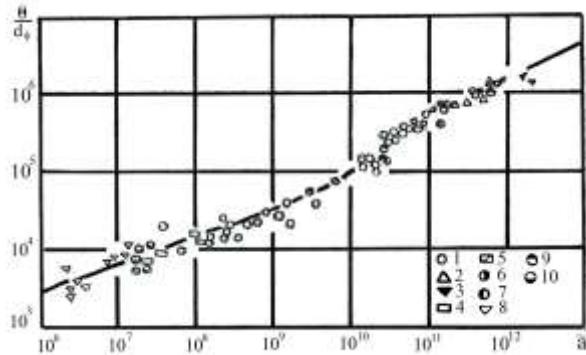


Figure: 1. Comparison of the measured and calculated by (7) values of the relative channel width:
 1-Indo-Pakistani canals; 2-Karakum canal; 3-Volga-Caspian Canal; 8.10-laboratory data; 5-Amu-Daria; 6-Tash-Saka; 7-Kyz-Ketken; 9-rivers of Belarus.

Belarus.

| источник | В м | Q м³/с | Измерительные данные | | | Расчётные значения ширин русла (по свободной поверхности) | | | | |
|--|--------|-----------|----------------------|----------|--------|---|----------------------|--------|------------------|-----------------|
| | | | Wcp м | hср м | % | d м | Санжар. Альбертск | Чечет. | Андреев. Б.С. | Михаил. А.Е. |
| Алтуник В.С. | 166,0 | 471,0 | 0,72 | 3,90 | 0,040 | 0,16 | 137,6 | 10,4 | 150 | 132,3 +151 |
| Караумуский и Волго- Каспийский каналы | 131,1 | 471,8 | 0,745 | 8,8 | 0,045 | 0,13 | 137,7 | 112,4 | 184,4 | +128,8 152 |
| | 148,2 | 475,0 | 0,39 | 5,46 | 0,024 | 0,15 | 138,2 | 122,9 | 184,4 | +148,0 153 |
| | 238 | 1950 | 1,24 | 6,10 | 0,040 | 0,17 | +280,0 | 217,9 | 334,1 | 392,2 281 |
| | 300 | 2010 | 0,67 | 10,0 | 0,013 | 0,17 | 284,2 | 272,3 | 390,0 | 347,1 +285 |
| | 251 | 1490 | 0,96 | 6,2 | 0,035 | 0,17 | 244,7 | 198,4 | 273,9 | 268,7 +250 |
| | 171 | 616 | 0,60 | 6,0 | 0,021 | 0,17 | 157,4 | 143,7 | 211,5 | 174,9 +169 |
| | 260 | 1470 | 0,6 | 8,7 | 0,017 | 0,17 | 243,1 | 227,1 | 306,7 | 233,6 248 |
| Чечет., реки и каналы | 120 | 270 | 1,364 | 1,65 | 4,500 | 2,05 | +104,2 | 161,9 | - | 4,4 40 |
| | 570 | 4196 | 1963 | 3,75 | 1,595 | 0,32 | +410,7 | 203,3 | 40,0 | 30,0 369 |
| США, | 183 | 2295 | 2206 | 5,43 | 2,170 | 4,77 | 303,7 | +226,6 | 212,2 | 25,4 95 |
| Пакистан | 282 | 378 | 1540 | 0,87 | 0,780 | 0,11 | 123,2 | 52,4 | 34,5 | 30,9 +143 |
| | 1380 | 6750 | 0,815 | 6,00 | 0,0167 | 0,01 | 500,9 | 285,4 | 337,7 | 359,2 +644 |
| Данные [7]. | 300 | 1300 | 0,87 | 5,00 | 0,048 | 0,20 | +228,6 | 176,9 | 186,7 | 120,9 +281 |
| Аму-Дарья | 110 | 170 | 0,70 | 2,20 | 0,041 | 0,15 | 82,7 | 63,0 | 74,8 | +121,9 97 |
| Таш-Сака | 60 | 150 | 0,85 | 3,00 | 0,082 | 0,15 | 77,6 | +59,9 | 107,7 | 89,9 92 |
| Кыз-Кеткен | | | | | | | | 3 | 2 | 0 |
| Кол-во с «*» | | | | | | | | | 4 | 7 |
| Примечание: знак «*» подразумевает включение соответствующего ряда | | | | | | | | | | |

To calculate the probable channel depth, dependence (8) is proposed. Comparison with the calculation results (Table 2) according to (8) shows that the correspondence is quite satisfactory for the task at hand.

Comparison of the values of the width of stable channels, calculated by the formulas of various authors.

Comparison of measured and calculated mean channel depths.

| Источник | п/п | Q м³/с | Измерительные данные | | Расчётная средняя глубина в соответствии с (2,4.10) | |
|---|-----|-----------|----------------------|----------|---|-------|
| | | | dcp м | hср м | hср м | |
| Алтуник В.С., Караумуский и Волго-Каспийский каналы [8] | 1 | 471,0 | 0,16 | 3,90 | | 4,17 |
| | 2 | 471,8 | 0,15 | 4,83 | | 4,24 |
| | 3 | 475,0 | 0,15 | 5,46 | | 4,25 |
| | 4 | 1950 | 0,17 | 6,10 | | 8,39 |
| | 5 | 2010 | 0,17 | 10,00 | | 8,52 |
| | 6 | 1490 | 0,17 | 6,20 | | 7,32 |
| | 7 | 616 | 0,17 | 6,00 | | 4,70 |
| | 8 | 1470 | 0,17 | 8,70 | | 7,28 |
| Данные [7] Аму- Дарья | 9 | 1300 | 0,20 | 5,00 | | 6,56 |
| | 10 | 170 | 0,15 | 2,20 | | 2,54 |
| | 11 | 150 | 0,15 | 3,00 | | 2,38 |
| Машкович К., Михнов А.Е., Мойкас Н.Л. [4], экспериментальные данные | 12 | 0,038 | 0,14 | 0,123 | | 0,110 |
| | 13 | 0,038 | 0,14 | 0,123 | | 0,110 |
| | 14 | 0,090 | 0,14 | 0,150 | | 0,165 |
| | 15 | 0,031 | 0,14 | 0,077 | | 0,099 |
| | 16 | 0,026 | 0,14 | 0,104 | | 0,91 |
| Викторова, поток [2] | 17 | 0,0044 | 0,20 | 0,034 | | 0,037 |

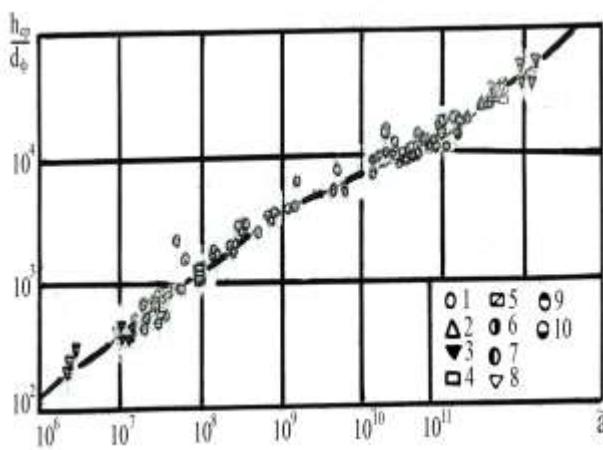


Figure: 2. Comparison of the measured and calculated by (8) values of the relative average depth of the channel (the legend is shown in Fig. 1)

Conclusions:

- As a result of the analysis of existing dependencies using the dimensional method, dependencies were obtained for calculating the width and average depth of large earth channels.
- The obtained dependencies can be used to design the parameters of the ground channels, i.e. to determine the width and depth of trapezoidal channels.

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